

EXPERIMENTAL STUDY OF LATERAL LOAD RESISTANCE OF UNCLASSIFIED TIMBER WALLS

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Abstract

Wood is used in many countries as a building material for low and high level constructions because this material has an excellent physical properties and environmentally friendly. Timber walls are the structural system that designed for purpose of resisting lateral loads and transmitting these forces to the foundations in a ductile behavior. According to the European Standard, the timber shear wall consists of timber frame and sheathing board connected together by fasteners. The sheathing board can be made from different materials such as Gypsum, Plywood, Fibre board and Oriented Strand Board (OSB). This paper is mainly focusing on mechanical behavior of unclassified timber walls under lateral loading (i.e. seismic and wind loads). Unclassified wooden planks have been used to construct the wall unit with three layers for each wall in cross perpendicular planks form. In this research an experimental study of timber walls were investigated in order to determine the lateral load resistance. This work is compared to (OSB) panels. An analytical modelling for the cross plank wall has been studied to investigate the behavior of this wall under the horizontal forces. Comparison between the theoretical modelling and the experimental results have been made for cross plank walls, on the other hand, a comparison between the Eurocode 5 calculations and experimental studies has been studied for (OSB) panels. Based on the data and results obtained from the experimental tests, this study confirms that the cross plank walls have more horizontal strength and rigidity than (OSB).

Keywords:

Unclassified wood, horizontal forces, cross plank walls, OSB panels, Analytical modelling, Eurocode 5.

1 INTRODUCTION

Timber walls are used in building as a structural members to resist the lateral forces and transmit it to the foundations in a ductile behavior. [Wei 2012]. Timber shear wall at European Standard EN 594, [EN 2006] consists of sheathing board connected by fasteners to the frame [Johan 2010]. Two methods have been presented in Eurocode 5 to calculate the lateral load carrying capacity of the wall, the first one is simple analytical approach and the second is based on the test procedures. Many studies have been conducted to study the load carrying capacity of timber walls by numerical and experimental methods especially on OSB. Elastic model with fully anchored sheathed has created to investigate the load carrying capacity by Girhammar and Kallsner [Kallsner 2009]. Other models have been proposed to investigate the horizontal resistance of the timber wall and investigated the effects of openings and the diameter of the diagonal member on the lateral resistance of the wall [Natalino 2016]. Previous studied confirmed that some parameters have a main role on the lateral load carrying capacity of the wall for instance, fixation of the foundation [Roberto 2013] opening size [Kozem 2012] and type of fasteners [Ozgun 2016]. Most of these parameters have been investigated experimentally. Previous studies confirmed that increasing the size of the

opening in timber walls will create stresses on the corner and will decrease the resistance of the wall. Some approaches have been developed by other authors to increase the load carrying capacity of the wall such as using stones [F. Vieux 2014] or adding diagonal member to the wall [P. Dobrila 2003]. The focus of this study is on the behavior of cross plank wall made from unclassified timber against horizontal forces. An experimental study has been done and the result compared with the behavior of Oriented Strand Board (OSB). Also an analytical model has been developed to investigate the ultimate load carrying capacity of the cross plank wall and also to calculate the top displacement of the wall.

2 ANALYTICAL MODELLING OF CROSS PLANK WALL UNIT

The cross plank wall is consists of cross vertical and horizontal planks, in each plane the plank could be in (horizontal or vertical) position. Each mesh of intersection carried out by pair of fasteners. The total number N of planks is constituted by an odd number N_v of vertical planks and an even or odd number N_H of horizontal planks

$$\begin{cases} N_v = (N + 1)/2 \\ N_H = (N - 1)/2 \end{cases}$$

A fold is formed by a number of planks (N_{px} for vertical planks and N_{py} for horizontal planks) forming its total width or height. The planks are characterized by a width e ($10 \leq e \leq 15$ cm) and a thickness d ($2 \leq d \leq 3$ cm), generally constant ($e_x = e_y = e$).

$$a \geq N_{px} e_x, \quad b \geq N d, \quad h \geq N_{py} e_y$$

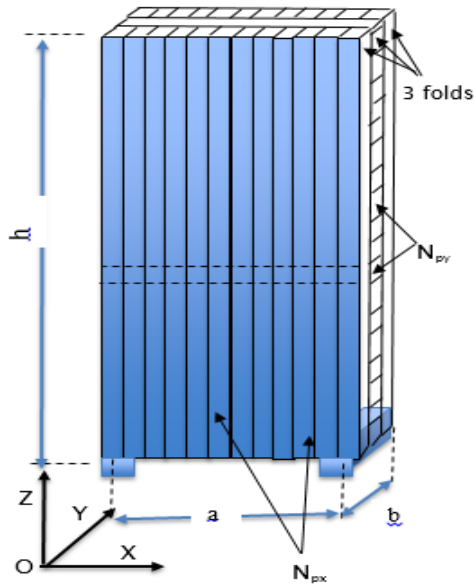


Fig 39: The formation of the cross plank wall unit.

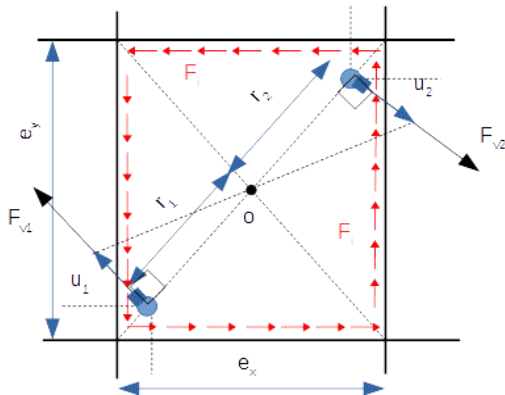


Fig 2: Shear plane intersection in the wall

The height of the wall varies from 2 to 3 m, its width (a) varies also from 1 to 1.2m and its thickness b depends on the total number N of the planks. Each mesh plank intersection is assembled by two fasteners ensuring the connection, in each case, between a vertical plank and a horizontal one. The fasteners are anchored respecting the edge distances of $5d$ (EC5) and are therefore located at a distance $r_1 = r_2 = r$ with respect to the geometrical center "o" of the mesh.

$$r_1 = r_2 = r = \sqrt{\left(\frac{e_x}{2} - 5d\right)^2 + \left(\frac{e_y}{2} - 5d\right)^2} = \frac{1}{\sqrt{2}}(e - 10d) \Big|_{e_x=e_y=e} \quad (1)$$

Where d is the diameter of the fasteners.

The manufacture of these walls is generally automated and the configuration of the assemblies on the whole of the meshes is almost homogeneous. The number

N_{m1} of meshes per shear plane and their total number N_m are given respectively by:

$$\begin{cases} N_{m1} = N_{px} N_{py} \\ N_m = N_{m1} N_v = N_{px} N_{py} \left(\frac{N+1}{2}\right) \end{cases}$$

The fasteners are subjected to the resistance due to the internal forces. The total number of fasteners is equal to $(2) \times (N_m)$ and each fastener has an ultimate force $F_{v,Rk}$ in single shear and can be calculated according to the Eurocode 5 as illustrated below:

$$F_{v,Rk} = \min \begin{cases} f_{h,1,k} \frac{t_1}{t_1} d & (a) \\ f_{h,2,k} \frac{t_2}{t_2} d & (b) \\ \frac{f_{h,1,k} \frac{t_1}{t_1} d}{1 + \beta} \left[\sqrt{\beta + 2\beta^2 \left[1 + \frac{2}{t_1} + \left(\frac{t_2}{t_1}\right)^2 \right] + \beta^3 \left(\frac{t_2}{t_1}\right)^2} - \beta \left(1 + \frac{2}{t_1} \right) \right] & (c) \\ 1,05 \frac{f_{h,1,k} \frac{t_1}{t_1} d}{2 + \beta} \left[\sqrt{2\beta(1 + \beta) + \frac{4\beta(2 + \beta)M_{y,Rk}}{f_{h,1,k} t_1^2 d}} - \beta \right] + \psi F_{ax,Rk} & (d) \\ 1,05 \frac{f_{h,1,k} \frac{t_2}{t_2} d}{1 + 2\beta} \left[\sqrt{2\beta^2(1 + \beta) + \frac{4\beta(1 + 2\beta)M_{y,Rk}}{f_{h,1,k} t_2^2 d}} - \beta \right] + \psi F_{ax,Rk} & (e) \\ 1,15 \sqrt{\frac{2\beta}{1 + \beta}} \sqrt{2M_{y,Rk} f_{h,1,k} d} + \psi F_{ax,Rk} & (f) \end{cases}$$

where:

- $\beta = f_{h,2,k}/f_{h,1,k}$ is the ratio between the characteristic embedment strength ($\beta = 1$)
- $M_{y,Rk}$ is the characteristic value of yield moment for fasteners.
- F_{ax} , is the characteristic axial withdrawal capacity of the fastener.

The lateral load carrying capacity of a cross-plank wooden wall is ensured by the rotational resistances of the crossover assemblies which develops a moment in ultimate resistance given by the following equations:

$$M_{v,Rk} = \sum_{i=1}^{N_m} [F_{v,Rk} (r_1 + r_2)]_i = F_{v,Rk} \sqrt{2} N_m (e - 10d) \Big|_{e_x=e_y=e} \quad (2)$$

Each fastener anchor in the wood can be represented by a spring K_{ser} , and the equilibrium of a crossing meshes is ensured by a rotational stiffness $K\theta$. By applying the energy conservation theorem, the rotational rigidity of each mesh of index "i" is represented as:

$$\frac{1}{2}(K_\theta \theta^2)_i = \frac{1}{2}K_{ser} (u_1^2 + u_2^2)_i = \frac{1}{2}K_{ser} (r_1^2 + r_2^2)_i = \frac{1}{2}K_{ser} (e - 10d)^2 \Big|_{e_x=e_y=e}$$

$$K_\theta = K_{ser} (r_1^2 + r_2^2)_i = K_{ser} (e - 10d)^2 \Big|_{e_x=e_y=e}$$

Consequently, the total rotational rigidity is obtained by the summation of the rotational rigidities as presented below:

$$K_\theta = \sum_{i=1}^{N_m} K_\theta = K_{ser} \sum_{i=1}^{N_m} (e - 10d)^2 \Big|_{e_x=e_y=e} = K_{ser} N_m (e - 10d)^2 \quad (3)$$

Four assumptions have been made for the distribution of the internal forces and the top displacement of the wall.

First assumption is based on the principle that the external force on the meshes is proportional to the rotational rigidities

$$F_i = \frac{K_\theta}{\sum_{i=1}^{N_m} K_\theta} F = \frac{K_\theta}{K_\theta} F; \Rightarrow M_\theta = F_i (e_x + e_y) = \frac{K_\theta}{K_\theta} (e_x + e_y) F = (e_x + e_y) \frac{F}{N_m} \Big|_{K_\theta = K_\theta}$$

Each nail has a force F_{vd} and can be calculate by:

$$F_{Vd} = \frac{M_{\theta}}{(r_1 + r_2)} = \frac{M_{\theta}}{\sqrt{2}(e-10d)} \Big|_{e_x=e_y=e} = \frac{\sqrt{2} e}{(e-10d)} \Big|_{e_x=e_y=e} \frac{F}{N_m} \Big|_{K_{\theta}=C^{\theta}} \leq F_{V,Rd} \quad (4)$$

Second assumption considers the static equilibrium between external and internal moments.

$$h F = \sum_{i=1}^{N_m} [F_{Vd} (r_1 + r_2)]_i = F_{Vd} \sum_{i=1}^{N_m} (r_1 + r_2)_i = F_{Vd} N_m \sqrt{2}(e-10d) \Big|_{e_x=e_y=e}$$

Then the force on each force can be calculated by:

$$F_{Vd} = \frac{h}{\sqrt{2}(e-10d)} \Big|_{e_x=e_y=e} \frac{F}{N_m} \leq F_{V,Rd} \quad (5)$$

Third assumption to calculate the displacement at the top of the wall by applying of the theorem of energy conservation between the work of the external force and those of the spiral springs of the different meshes of crossing.

$$F \Delta = \frac{1}{2} \sum_{i=1}^{N_m} K_{\theta} \theta_i^2 = \frac{K_{\theta}}{2} \Big|_{K_{\theta}=C^{\theta}} \times \left(\frac{\Delta}{h} \right)_{\theta_i=\theta}^2$$

$$\Delta = \frac{2h^2}{K_{\theta}} F \quad (6)$$

Fourth assumption is made to calculate the ultimate When considering the ultimate single shear strength of the nail, the ultimate characteristic force that can be applied to the wall can be presented as:

$$M_{v,Rk} = F_{U,k} h = F_{v,Rk} \sqrt{2} N_m (e-10d) \Big|_{e_x=e_y=e}$$

$$F_{U,k} = \sqrt{2} N_m \frac{(e-10d)}{h} \Big|_{e_x=e_y=e} F_{v,Rk} \quad (7)$$

3 EXPERIMENTAL INVESTIGATION

3.1 Compressive characteristic tests of wood

Nine samples of wood were tested for compressive strength to classify the type of wood and to determine the characteristics of wood material, five samples were tested in parallel to the grain of wood and four samples were tested in transverse to gain of wood. The average compressive strength parallel to grain of wood ($f_{c,0,k}$) was 29.9 MPa as illustrated in figure 3 and the average compressive strength perpendicular to the gain of wood ($f_{c,90,k}$) was 4.2 MPa as mentioned in figure 4. Taking in to account the compressive strength and the mean density of the material ($\rho = 463.4 \text{ Kg/m}^3$) and comparing these three values with wood classification of **Eurocode (5) NF EN 1194**, the closest class of this wood material is C30.

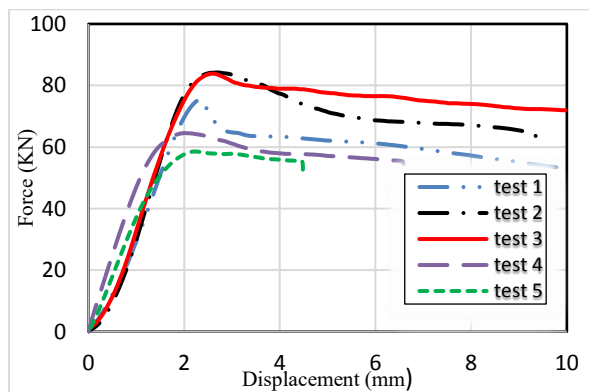


Fig 3: wood compressive strength Parallel to the grain

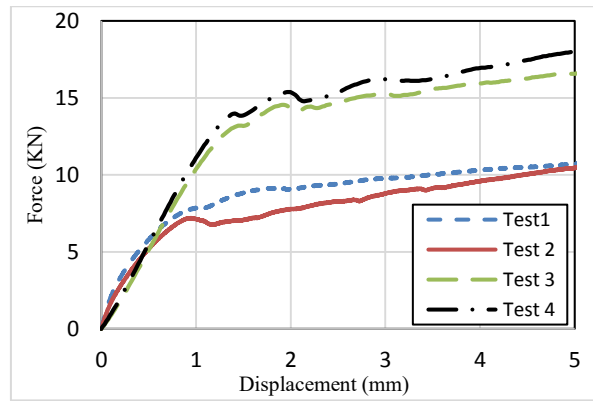


Fig 4: wood compressive strength transverse to the grain.

3.2 Load carrying capacity tests for fasteners

Three tests were examined to determine the load carrying capacity of fasteners, double shear timber-to-timber with three pieces connected using two nails per side. The nails are rounded cross-section 2.5 mm of diameter and length 45 mm were used. The mean value of the maximum load for fastener $F_{v,R,mean}$ was 1.59 KN.

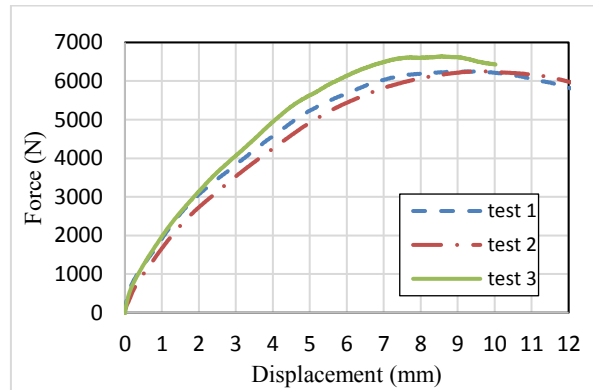


Fig 5: Simple shear behavior of fasteners.



Fig 6: Shear test set up for fasteners.

3.3 Racking tests of cross plank wall

Five walls of large scale timber cross planks were used, each wall of cross planks consists of three layers and the connection of these layers is done using rounded nails of diameter 2.5 mm and 45 mm long. The walls were 250 cm in height, 120 cm in width and 7.5 cm in thick. A horizontal racking load was applied to the top of wall by a hydraulic jack.

3.4 Racking tests of Oriented Strand Board (OSB) panel

Timber frame with oriented strand board was used with height 300 cm, width 121cm and thickness 13.5 cm. The wall was fixed to the floor to avoid sliding, a horizontal load was applied to the top of the panel with the same previous techniques. The wall was consisting of three columns (studs) and OSB3 sheet.



Fig 7: Timber cross plank wall in test.

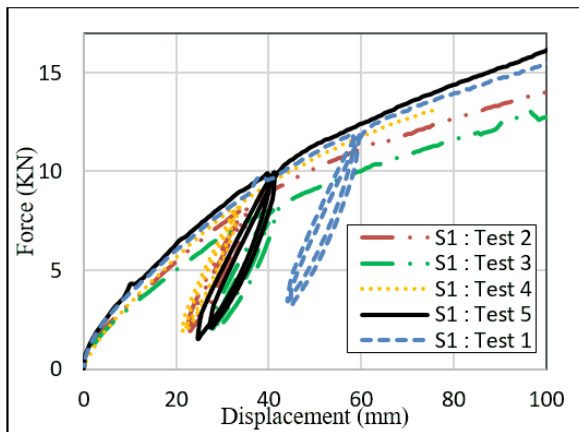


Fig 8: Force-Displacement behaviour.



Fig 9: OSB panel in test.

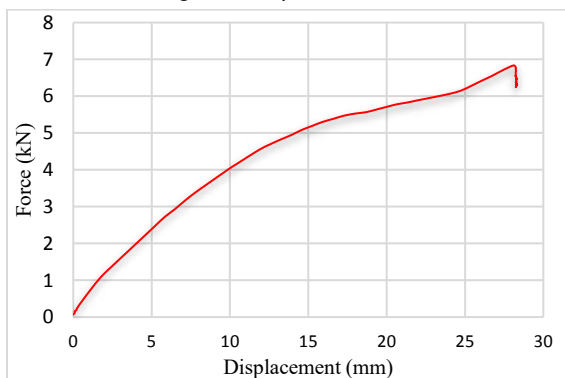


Fig 10: Force-Displacement behavior for OSB panel.

4 CALCULATIONS AND DISCUSSION

The lateral load carrying capacity ($F_{v,RK}$) for the nails used in cross plank wall has been calculated according to the Eurocode 5 formulas and it was 616.66 N. The ultimate horizontal force at the top of the wall has been calculated based on the proposition 4 and it was 11.16 KN with a top displacement 50.40 mm. considering the factor of safety equal to 1.3 then the design force of the wall will be $11.16/1.3 = 8.58$ KN. The internal force on each nail as a proportional of the rotational rigidities and based on the first proposition was $F_i = 74$ N but considering to the second assumption of moments equilibrium, the internal force at the nail was 616.66 N.

Tab. 1: assumption values for cross plank wall

	Proposition 1	Proposition 2	Proposition 3	Proposition 4
F_{vd} (N)	74 N	616.66 N	----	----
Δ (mm)	----	----	50.40 mm	----
$F_{u,k}$ (kN)	----	----	----	11.16 kN

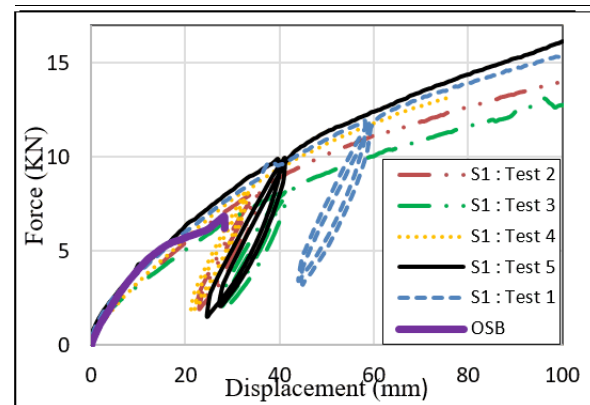


Fig 11: Comparison between the behavior of cross plank walls and OSB panel.

5 CONCLUSION

The behavior of cross plank walls made from unclassified timber proved a greater resistance and more rigidity than OSB panel due to the huge numbers of internal rotational moments that created in each shear plane in the wall. An analytical model has been created to represent the internal forces on each fastener and calculating the ultimate force on the top of the wall. This wall has a good performance in construction of civil engineering to resist the lateral loads.

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